

Introduction to the Finite Element Method

Sankara J. Subramanian

Department of Engineering Design
Indian Institute of Technology, Madras

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What is FEM?

Basic
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Basic
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Implementation

Shape functions

FE matrices

FE equations

Advanced FEM

Nonlinearity

Quasi-continuum

Conclusion

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- FE matrices
- FE equations

3 Advanced FEM

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The Finite Element Method is a numerical method used for solving field problems

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■ Origins in structural mechanics

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- Origins in structural mechanics
- Has strong mathematical foundation

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- Origins in structural mechanics
- Has strong mathematical foundation
- Widely used by researchers – structural and solid mechanics, fluid flow, heat transfer, electricity and magnetism and various coupled problems

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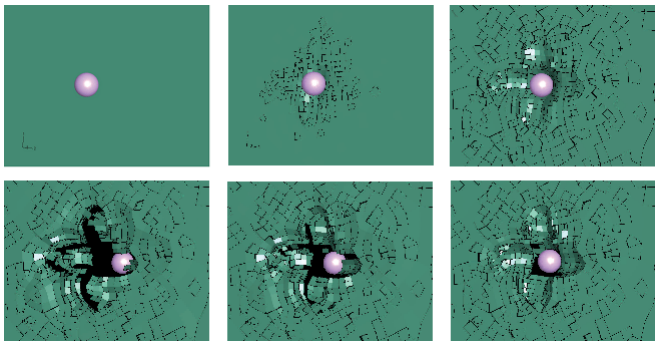
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- Origins in structural mechanics
- Has strong mathematical foundation
- Widely used by researchers – structural and solid mechanics, fluid flow, heat transfer, electricity and magnetism and various coupled problems
- Routinely used in the industry – design of buildings, airframes, electric motors, automobiles, materials

Most problems of interest to engineers and scientists cannot be solved analytically

From 'Projectile Impact on a Carbon Fiber Reinforced Plate', Abaqus Technology Brief, Simulia Corporation, 2007



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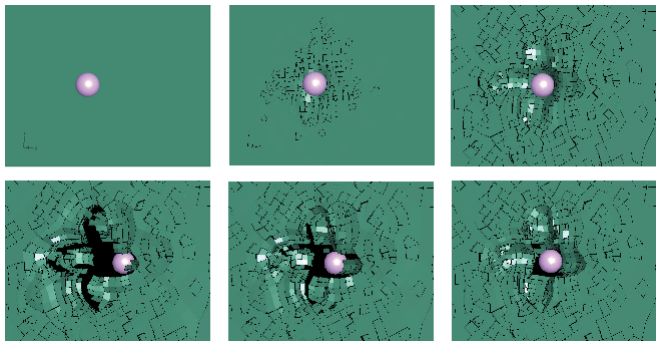
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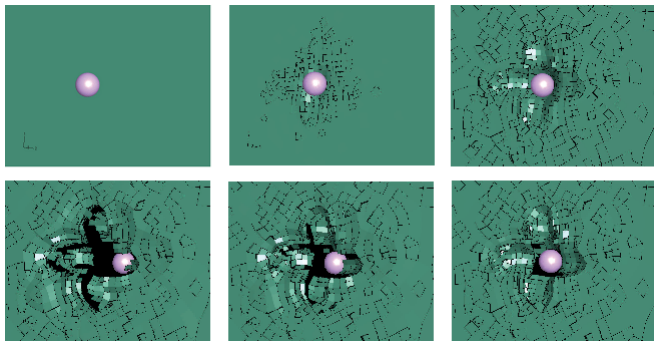
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- Plate is **heterogeneous**

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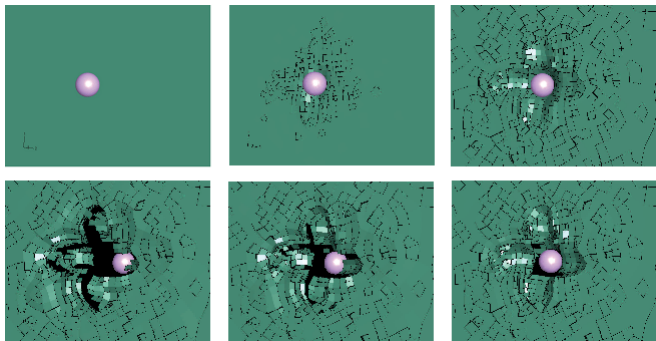
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- Plate is **heterogeneous**
- **Contact** areas between steel ball and plate not known *a priori*

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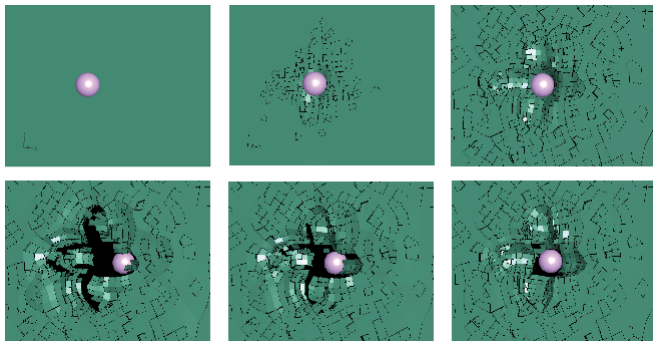
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- Plate is **heterogeneous**
- **Contact** areas between steel ball and plate not known *a priori*
- **Failure** criteria are complex

Most problems of interest to engineers and scientists cannot be solved analytically

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- Plate is **heterogeneous**
- **Contact** areas between steel ball and plate not known *a priori*
- **Failure** criteria are complex
- Analytical solution is out of the question!

Governing differential equations are converted to algebraic equations

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- FEM is used to solve governing differential equations **approximately**
- ODEs or PDEs are converted to a (large) system of algebraic equations, solved on computers

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- Quality of solution improves with increasing number of elements
- 100 unknowns large in 1960s, now 10^6 unknowns routine!

We are interested in calculating the displacements and stresses everywhere in a deforming elastic body

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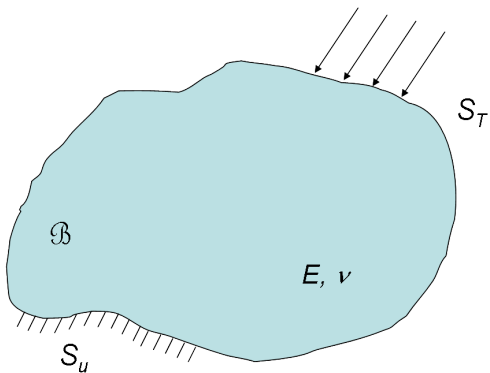
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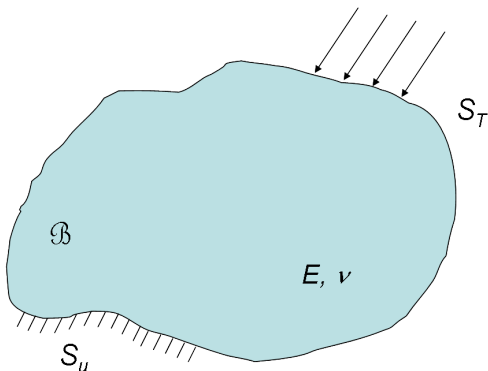
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- Elastic deformation is reversible \implies body returns to original configuration when loads are removed

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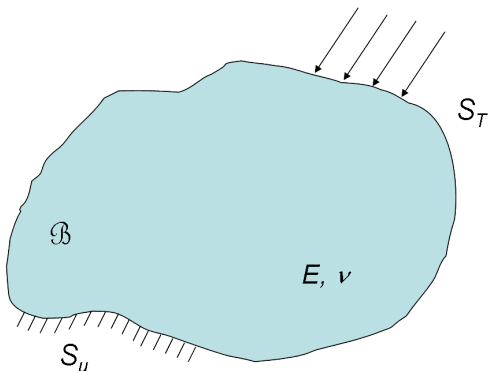
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- Elastic deformation is reversible \implies body returns to original configuration when loads are removed
- We know the forces and displacements imposed on the body

We are interested in calculating the displacements and stresses everywhere in a deforming elastic body

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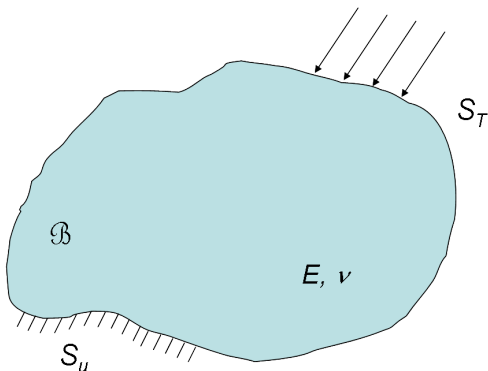
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- Force F applied to a bar of cross-section A , stress $\sigma = F/A$

We are interested in calculating the displacements and stresses everywhere in a deforming elastic body

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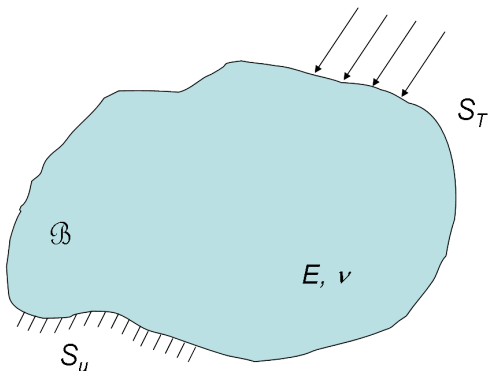
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- We know the forces and displacements imposed on the body
- Force F applied to a bar of cross-section A , stress $\sigma = F/A$
- **For arbitrary 3-D solids, how do we compute the displacements and stresses everywhere in the body?**

The elasticity boundary value problem is specified by the governing equation 1, ...

The governing equation (no body forces, quasi-static case) is the **linear momentum balance** equation:

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The governing equation (no body forces, quasi-static case) is the **linear momentum balance** equation:

- The stress tensor quantifies intensity of force (force/unit area) at a point.

The elasticity boundary value problem is specified by the governing equation 1, ...

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$$\frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{12}}{\partial x_2} + \frac{\partial \sigma_{13}}{\partial x_3} = 0$$

$$\frac{\partial \sigma_{21}}{\partial x_1} + \frac{\partial \sigma_{22}}{\partial x_2} + \frac{\partial \sigma_{23}}{\partial x_3} = 0$$

$$\frac{\partial \sigma_{31}}{\partial x_1} + \frac{\partial \sigma_{32}}{\partial x_2} + \frac{\partial \sigma_{33}}{\partial x_3} = 0$$

1

σ_{ij} - stress tensor components,

The elasticity boundary value problem is specified by the governing equation 1, ...

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- The stress tensor quantifies intensity of force (force/unit area) at a point.

$$\begin{aligned}\frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{12}}{\partial x_2} + \frac{\partial \sigma_{13}}{\partial x_3} &= 0 \\ \frac{\partial \sigma_{21}}{\partial x_1} + \frac{\partial \sigma_{22}}{\partial x_2} + \frac{\partial \sigma_{23}}{\partial x_3} &= 0 \\ \frac{\partial \sigma_{31}}{\partial x_1} + \frac{\partial \sigma_{32}}{\partial x_2} + \frac{\partial \sigma_{33}}{\partial x_3} &= 0\end{aligned}\quad \boxed{1}$$

σ_{ij} - stress tensor components,

- a system of 3 coupled PDEs for 6 unknown stress components ($\sigma_{ij} = \sigma_{ji}$)

The elasticity boundary value problem is specified by the governing equation 1, ...

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σ_{ij} - stress tensor components,

- a system of 3 coupled PDEs for 6 unknown stress components ($\sigma_{ij} = \sigma_{ji}$)

$$\boxed{\sigma_{ij,j} = 0, \quad i = 1, 3}$$

... boundary conditions 2, ...

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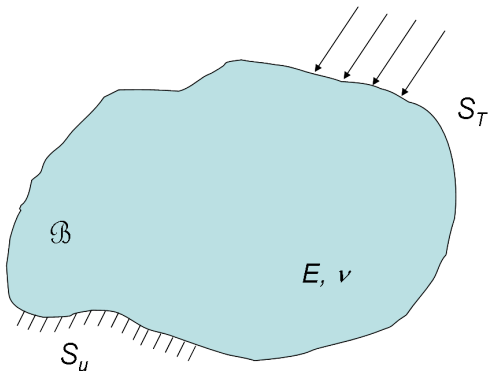
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$$\sigma_{ij}n_j = T_i = T_i^{\text{sp}} \text{ on } S_T$$

$$u_i = u_i^{\text{sp}} \text{ on } S_u$$

2

S_T and S_u are the portions of external surface S on which forces and displacements are specified respectively.

... constitutive equations 3 ...

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Constitutive Equation - Hooke's Law for isotropic, linear elasticity:

$$\sigma_{ij} = \frac{E}{1 + \nu} \left[\epsilon_{ij} + \frac{\nu}{1 - 2\nu} \epsilon_{kk} \delta_{ij} \right] \quad \boxed{3}$$

E - Young's modulus, ν - Poisson's ratio, ϵ_{ij} - strain tensor components, δ_{ij} - Kronecker delta

Strain-displacement relations:

Strain-displacement relations:

$$\epsilon_{ij} = \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) \quad \boxed{4}$$

Strain-displacement relations:

$$\epsilon_{ij} = \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) \quad \boxed{4}$$

$$\epsilon_{11} = \frac{\partial u_1}{\partial x_1}$$

reduces to the familiar definition of $\epsilon = \Delta l / l$ for the 1-D case.

The principle of virtual work is the basis of FEM

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- The requirement of satisfying the governing PDE pointwise is relaxed: we seek to satisfy it in a *weak or integrated form*.

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- Instead of satisfying point-wise the equilibrium equations

$$\sigma_{ij,j} = 0, \quad i = 1, 3,$$

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- Instead of satisfying point-wise the equilibrium equations

$$\sigma_{ij,j} = 0, \quad i = 1, 3,$$

- we solve

$$\int_V u_i^* \sigma_{ij,j} dV = 0$$

Finite element equations follow from PVW

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- Using the divergence theorem and the strain-displacement relations, we can restate the integral as

$$\int_V \epsilon_{ij}^* \sigma_{ij} dV = \int_S F_i u_j^* dS$$

where F_i are components of the applied loads on the boundary

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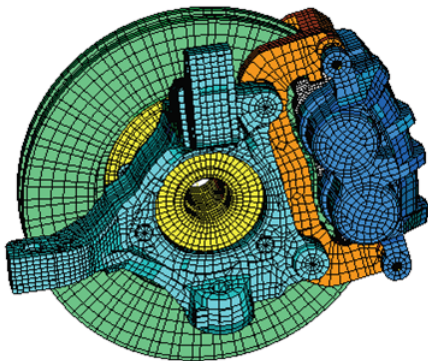
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$$\int_V \epsilon_{ij}^* \sigma_{ij} dV = \int_S F_i u_j^* dS$$

where F_i are components of the applied loads on the boundary

- This is not an energy balance, it is merely a restatement of equilibrium!

How is a finite-element solution implemented?



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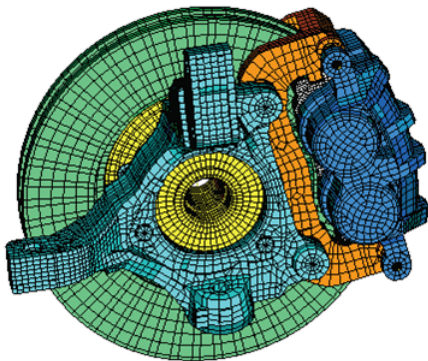
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How is a finite-element solution implemented?



- First, the solid is *discretized* into a number of elements ('meshing')

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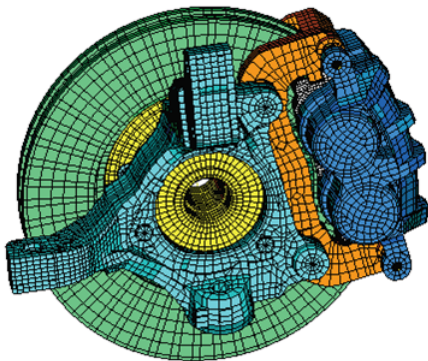
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- First, the solid is *discretized* into a number of elements ('meshing')
- Nodes are typically points on the element boundaries

Displacement at any point in the solid is expressed in terms of nodal displacements

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- The actual displacement components at any point inside an element are interpolated from the (unknown) actual nodal values \mathbf{u}^i

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- The actual displacement components at any point inside an element are interpolated from the (unknown) actual nodal values \mathbf{u}^i
- The virtual displacements are also interpolated in the same way

$$\mathbf{u}(x_1, x_2, x_3) = \sum_{i=1}^3 N^i(x_1, x_2, x_3) \mathbf{u}^i$$

N^i are **shape functions**

Displacement at any point in the solid is expressed in terms of nodal displacements

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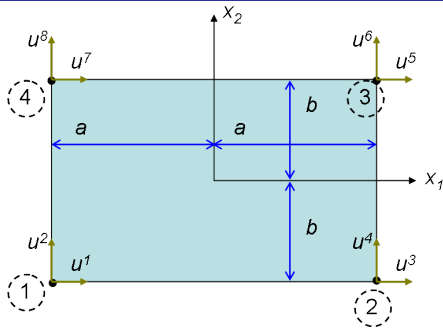
- The actual displacement components at any point inside an element are interpolated from the (unknown) actual nodal values \mathbf{u}^i
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$$\mathbf{u}(x_1, x_2, x_3) = \sum_{i=1}^3 N^i(x_1, x_2, x_3) \mathbf{u}^i$$

N^i are **shape functions**

- For full 3D analysis, ($3 \times \#$ of nodes) unknowns

Linear shape functions are the simplest



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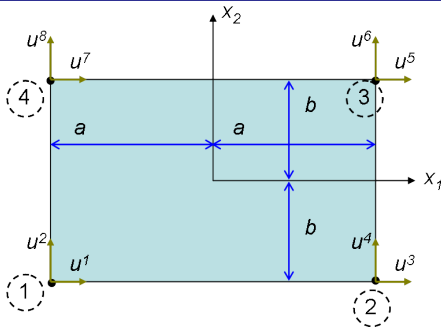
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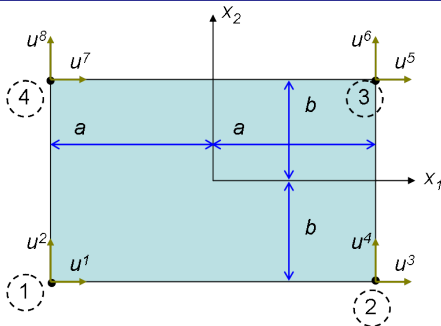
Linear shape functions are the simplest



$$u_1(x_1, x_2) = \sum_{i=1}^4 N^i(x_1, x_2) u^{2*i-1}$$

$$u_2(x_1, x_2) = \sum_{i=1}^4 N^i(x_1, x_2) u^{2*i}$$

Linear shape functions are the simplest



$$u_1(x_1, x_2) = \sum_{i=1}^4 N^i(x_1, x_2) u^{2*i-1}$$

$$u_2(x_1, x_2) = \sum_{i=1}^4 N^i(x_1, x_2) u^{2*i}$$

$$N^1 = \frac{(a-x_1)(b-x_2)}{4ab}$$

$$N^2 = \frac{(a+x_1)(b-x_2)}{4ab}$$

$$N^3 = \frac{(a+x_1)(b+x_2)}{4ab}$$

$$N^4 = \frac{(a-x_1)(b+x_2)}{4ab}$$

Displacements are written in vector form

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$$\begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} = \begin{bmatrix} N^1 & 0 & N^2 & 0 & N^3 & 0 & N^4 & 0 \\ 0 & N^1 & 0 & N^2 & 0 & N^3 & 0 & N^4 \end{bmatrix} \begin{Bmatrix} u^1 \\ u^2 \\ u^3 \\ u^4 \\ u^5 \\ u^6 \\ u^7 \\ u^8 \end{Bmatrix}$$

Derivatives of shape functions relate strains to displacements

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Recall strain-displacement relations:

$$\epsilon_{ij} = \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right)$$

In vector form,

$$\boldsymbol{\epsilon} = \left\{ \begin{array}{c} \epsilon_{11} \\ \epsilon_{22} \\ \epsilon_{12} \end{array} \right\} = [\mathbf{B}]\{\mathbf{u}^i\}$$

where

$$[\mathbf{B}] = \begin{bmatrix} \partial/\partial x_1 & 0 \\ 0 & \partial/\partial x_2 \\ (1/2) \partial/\partial x_1 & (1/2) \partial/\partial x_2 \end{bmatrix} [\mathbf{N}]$$

Matrix of elastic constants relates stress components to strain components

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Recall Hooke's law:

$$\sigma_{ij} = \frac{E}{1 + \nu} \left[\epsilon_{ij} + \frac{\nu}{1 - 2\nu} \epsilon_{kk} \delta_{ij} \right]$$

In matrix form, we can write this as

$$\{\boldsymbol{\sigma}\} = [\mathbf{D}]\{\boldsymbol{\epsilon}\} = [\mathbf{D}][\mathbf{B}]\{\mathbf{u}^i\}$$

where $[\mathbf{D}]$ is a matrix of elastic constants

Substitution into PVW gives element equilibrium equations

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Recall the PVW:

$$\int_V \epsilon_{ij}^* \sigma_{ij} dV = \int_S F_i u_j^* dS$$

In matrix form,

$$\int_V \{\epsilon^*\}^T \{\sigma\} dV = \int_S \{\mathbf{u}^*\}^T \{\mathbf{F}\} dS$$

$$[\mathbf{K}]\{\mathbf{u}^i\} = \{\mathbf{R}\}$$

where

$$[\mathbf{K}] = \int_V [\mathbf{B}]^T [\mathbf{D}] [\mathbf{B}] dV \quad \text{and} \quad \{\mathbf{R}\} = \int_S [\mathbf{N}]^T \{\mathbf{F}\} dS$$

Assembly of element contributions yields global stiffness equations

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- Global equations are solved for the nodal displacements in the entire solid

Assembly of element contributions yields global stiffness equations

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- Global equations are solved for the nodal displacements in the entire solid
- Using the finite-element matrices, strains and stresses are then computed

FEM is a versatile tool for studying nonlinear and coupled phenomena

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- Nonlinear material behaviour: plasticity, creep

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- Nonlinear material behaviour: plasticity, creep
- Large deformations: Geometric non-linearity

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- Contact: bodies come into contact as a result of load application

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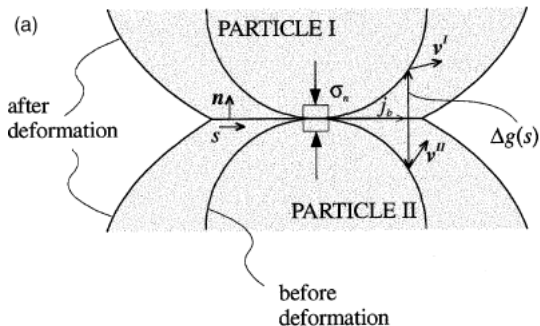
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- Large deformations: Geometric non-linearity
- Contact: bodies come into contact as a result of load application
- Coupled phenomena: e.g. Piezoelectric ceramics, hydrogen embrittlement
- Fatigue and fracture studies

A finite-element formulation was used to study hot-isostatic pressing of powders



- Bulk deformation: Elasticity + power-law creep
- Interparticle mass diffusion
- Surface diffusion

Modeling the interaction between densification mechanisms in powder compaction (2001), S. J. Subramanian and P. Sofronis, Int. J. Sol. Struct., 38, 7899-7918

FEM has recently been coupled with atomistic models

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- Stress-strain relations are derived from atomistic calculations instead of traditional continuum descriptions

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- Stress-strain relations are derived from atomistic calculations instead of traditional continuum descriptions
- Can capture the physics extremely accurately, but can only model very small volumes (100s of nm^3)

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- <http://www.qcmethod.com>: a good resource

Quasi-continuum method used to study crack propagation in Ni

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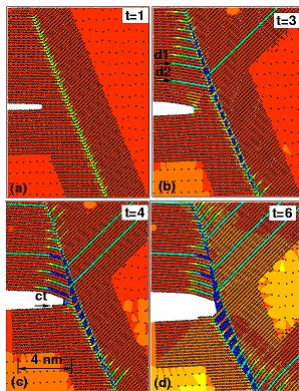
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Reference: *Quasicontinuum simulation of fracture at the atomic scale (1998)*, R. Miller, E. B. Tadmor, R. Phillips and M. Ortiz, *Modelling and Simulation in Materials Science and Engineering*, vol. 6, 607-638

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